



SYMBOLS:

Ser - service limit state I Str - strength limit state I Ext - extreme event limit state I B' - effective footing width (ft)

q'o - net bearing stress (ksf), OG assumed to be FG at toe

qo - gross uniform bearing stress (ksf)

TABLE OF REINFORCING STEEL, DIMENSIONS AND DATA					
DESIGN H	4'	6′	8′	10'	12′
W	8'-4"	9'-3"	10'-3"	11'-0"	12'-4"
F SPREAD FOOTING	1'-4"	1'-4"	1'-4"	1'-4"	1'-7"
BATTER	NONE	NONE	NONE	100:3	100:5
@ BARS	#5 @ 16	#5 @ 16	#5 @ 16	#5 @ 16	#5 @ 16
BARS BAR	NONE	NONE	#5 @ 16	#5 @ 16	#5 @ 16
© BARS	#6 @ 8	#7 @ 8	#8 @ 8	#9 @ 8	#9 @ 8
Ser: B', q'o	5.6, 1.4	6.4, 1.8	7.4, 2.2	7.8, 2.6	8.9, 3.0
Str: B', go	3.6, 2.4	4.2, 3.0	5.0, 3.4	5.3, 4.0	6.4, 4.2
Ext: B', qo	4.4, 2.1	4.2, 3.0	4.2, 4.0	3.9, 5.5	4.2, 6.7

DESIGN CONDITIONS:

Design H may be exceeded by 6" before going to the next size. Special footing design is required where foundation material is incapable of supporting bearing stress listed in the table.

COUNTY

April 20, 2012

PLANS APPROVAL DATE

ROUTE

Jan Dong

TO ACCOMPANY PLANS DATED

POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS

Gary Wang

C58298

op. 6-30-12

CIVIL

DESIGN NOTES:

DESIGN: AASHTO LRFD Bridge Design Specifications, 4th Edition with California Amendments LS: Varied surcharge on level ground surface

Stem Architectural Treatment of thickness up to 6" of concrete (75 psf) considered DC:

SEISMIC: $k_h = 0.2 \\ k_V = 0.0$ $\emptyset = 34^{\circ}$ $\gamma = 120 \text{ pcf}$ SOIL:

REINFORCED CONCRETE:

f'c = 3,600 psify = 60,000 psi

LOAD COMBINATIONS AND LIMIT STATES: Service I 0 = 1.00DC+1.00EV+1.00EH+1.00LS Strength I 0 = 0C+0EV+0EH+1.75LS Extreme I 0 = 1.00DC+1.00EV+1.00EH+1.00ED+1.00EQE

Where:

Force Effects
1.25 or 0.90, Whichever Controls Design
1.35 or 1.00, Whichever Controls Design
1.50 or 0.90, Whichever Controls Design
Dead Load of Structure Components
Horizontal Earth Fill Pressure Q: q: p: n: DC: EH: EV: LS: EQE:

Vertical Earth Pressure from Earth Fill Weight Live Load Surcharge Seismic Earth Pressure

Soil and Structural and Nonstructural Components Inertia

NOTES:

1. At (a) and (b) bars:

H ≤ 6', no splices are allowed within 1'-8" above the top of footing. H > 6', no splices are allowed within H/4 above the top of footing.

STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION

RETAINING WALL TYPE 5 (CASE 3)

NO SCALE

RSP B3-4C DATED APRIL 20, 2012 SUPPLEMENTS THE STANDARD PLANS BOOK DATED 2010.

REVISED STANDARD PLAN RSP B3-4C